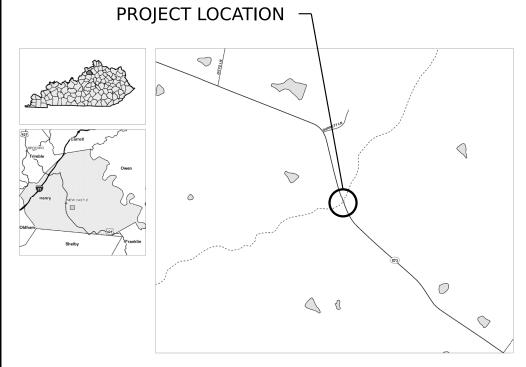
# TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

# HENRY COUNTY S. PROPERTY ROAD KY 573 OVER TRIB TOWN CREEK STA. 10+14.00



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BID ITEM	Concrete Class "A"	⊖ Concrete Class "AA"	Concrete Sealing	Masonry Coating	Steel Reinforcement, Epoxy Coated	Steel Reinforcement	Structural Steel	Shear Connector	Remove Superstructure	Remove Concrete Masonry	Armored Edge for Concrete	Deck Drain	Rail System Single Slope 40 Inch	Thrie Beam Guardrail Transition TL-3	Guardrail Steel "W" Beam (Single Face)	Guardrail End Treatment Type 1	Roadway Excavation	Asphlat Seal Coat	В
UNIT	C.Y.	C.Y.	S.F.	S.Y.	LBS.	LBS.	L.S.	L.S.	L.S.	C.Y.	L.F.	Each	L.F.	Each	L.F.	Each	C.Y.	Ton	╽╠
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BID ITEM	Edge Key	Cyclopean Stone Rip Rap	Foundation Preparation	Staking	Temporary Signs	Maintain and Control Traffic	Barricade Type III	Demobilization	Mobilization- For Concrete Surface Treatment	Ditching And Shouldering	Crushed Stone Base	டுCL3 ASPH BASE 1.00D PG64-22	டுCL3 ASPH SURF 0.38D PG64-22	Object Marker TY 3	Longitudinal Edge Key	Ashpalt Material For Tack	Cement Stabilized Roadbed	Asphalt Seal Aggregate	F
UNIT	L.F.	Tons	L.S.	L.S.	S.F.	L.S.	Each	L.S.	L.S.	L.F.	Ton	Ton	Ton	Each	L.F.	Ton	S.Y.	Ton	1 ⊦
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BRIDGE TOTALS	40	44	1	1	290	1	2	1	1	180	161	17	30	4	150	0.2	159	2	]

- ① Class "AA" Concrete above bridge seats is included in Superstructure Quantities
- ② Approximate Weight of Structural Steel = 12934 lbs.
- 3 Approximate Number of Shear Connectors = 660
- 4 Estimated at 115 lbs Per SQ. YD. per Inch of Depth
- (5) Estimated at 110 lbs Per SQ. YD. per Inch of Depth
- 6 Estimated 0.84 lbs per SQ. YD.

# S1 Title Sheet S2-S3 General Notes S4 Layout S5 Concrete Removal Details S6-S7 Abutment #1 S8-S9 Abutment #2 S10 Framing Plan 511-S12 Superstructure S13 Sepia 048 - Joint Waterproofing S14 Construction Elevations S15 Detour Details S16 Typical Roadway Sections SPECIAL NOTES Special Note for Concrete Sealing Special Note for Shoulder Preparation Special Note for Edge Key **SPECIAL PROVISIONS** STANDARD DRAWINGS BGX-015-04 Bridge Drains GGX-006-10 Stencils for Structures GX-012-02 Geotechnical Legend BJE-001-14 Armored Edges Railing System 40 Inch Single Slope Thrie Beam Transition Guardrail End Treatment Type 1 Steel Beam Guardrail (W Beam Guardrail Components Steel Guardrail Posts Installation of Guardrail End Treatment Type 1 **SPECIFICATIONS** 2019 Standard Specifications for Road and Bridge Construction.

**INDEX OF SHEETS** 

2020 AASHTO LRFD Bridge Design Specifications

COMMONWEALTH OF KENTUCKY K DEPARTMENT OF HIGHWAYS

REVISION DAT

Division of Structural Design 
 DATE:
 DECEMBER 2024
 CHECKED BY
 TITLE SHEET

 DESIGNED BY:
 N. CORDTZ
 L. LIKINS
 CROSSING

 DETAILED BY:
 M. BAWITHAWNG
 N. CORDTZ
 TRIB TO TOWN CREEK

ROUTE BRIDGE ID. COUNTY OF 052B00043N HENRY

KY 573 SHEET NO. DRAWING NUMBER S1 28958

SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction with current Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN LOAD: This bridge slab is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by 25%. The beams are designed for a HS-25 Live Load.

FUTURE WEARING SURFACE: This structure is designed for a 60 PSF future wearing surface load.

DESIGN STRESSES: Concrete Class "A" ~ f'c = 3500 psi

Concrete Class "AA" ~ f'c = 4000 psi Steel Reinforcement ~ Fy = 60,000 psi Structural Steel Yield Strength ~ Fy = 50,000 psi

DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specifications. The steel beams and upgrades were designed with the Load Factor Method specified in the 17th edition AASHTO Standard Specifications for Highway Bridges.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Any reinforcement bars designated by suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters.

BEVELED EDGES: Bevel all exposed edges 3/4" unless otherwise noted.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

FOUNDATION DATA: See Foundation Layout Sheet.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

SUPERSTRUCTURE SLAB: Ensure the entire superstructure slab and diaphragms are poured continuously, out to out, before allowing any concrete to set.

MASONRY COATING: Apply a masonry coating finish to the substructure in accordance with section 601.03.18

CONCRETE SEALER: The superstructure deck, barriers and overhangs shall also be sealed as shown herein these plans. Concrete surfaces (except the deck) shall receive the ordinary surface finish as described in section 601.03.18(A) prior to being sealed. Seal deck in accordance with the special note for concrete sealing.

CONCRETE: Class "AA" is to be used throughout the new superstructure. Class "A" is to be used on the substructures.

SLOPE PROTECTION: Use dry cyclopean stone slope protection in accordance with the plans and Specifications. Geotextile Fabric is to be incidental to this item.

FORM WEIGHT: Design Includes 16 psf for stay in place firm weight and allows for concrete filling voids. If contractor chooses to fill flutes with concrete, cost for extra concrete is incidental.

ON-SITE INSPECTION: Each contractor submitting a bid for this work shall make a thorough inspection of the project site prior to submitting a bid and shall be thoroughly familiarized with existing conditions so that work can be expeditiously performed after a contract is awarded. Submission of a bid will be considered evidence of this inspection having been made. Any claims resulting from site conditions will be honored by the Department of Highways.

DRILLING AND GROUTING: In accordance with Section 826 of the specifications, drill holes to a depth as shown herein these plans and apply a Type IV epoxy bonding adhesive in the holes. All costs associated with this work shall be incidental to the unit price bid for Class "A" Concrete.

BONDING CONCRETE TO PREVIOUS POURED CONCRETE: Use an epoxy bond coat as described in section 511.03.02 of the Standard Specifications to bond the new concrete to the existing concrete at all construction joints noted in the plans. Include the cost of this work in unit price bid for Concrete "A".

EXISTING REINFORCING STEEL: The costs of cutting, bending and cleaning existing reinforcing steel in the abutments, if required, is to be incidental to the lump sum bid for "Remove Superstructure".

MAINTAIN AND CONTROL TRAFFIC: The contractor is fully responsible for maintaining and controlling traffic on this project. Bridge is to be fully closed to traffic for construction. Contractor shall provide signs letting public know of bridge closure for each direction and shall place type III barricades at each end of the bridge.

# **GENERAL NOTES**

BRIDGE OVERLAY APPROACH PAVEMENT: Excavation into existing pavement or ground behind abutment may be required for abutment rehab. Include all costs for excavation in the lump sum price bid for Remove Superstructure. Backfill of excavated soil shall be with geotextile wrapped 57s. Pavement structure in the fully removed sections shall consist of a minimum of 8" cement stabilized road bed, 16" crushed stone base, 2~3" minimum lifts of asphalt base, and 1.5 inch minimum of asphalt surface. Surface shall be placed over existing pavement where existing pavement was not fully removed to provide a smooth transition from the bridge deck elevation to the existing pavement elevation within 75 feet off the end of the bridge. Provide plan to Engineer for approval prior to beginning work.

FIELD MEASUREMENTS: All dimensions and elevations given in these plans are based on field measurements. Prior to beginning work or ordering any materials, the contractor shall verify all dimensions and elevations. No claim will be honored by the Department of Highways regarding site conditions.

DAMAGE TO THE SUBSTRUCTURES: The contractor is responsible for any and all damages to the existing substructures during reconstruction even to the replacement of the entire substructure, should they be damaged due to their actions.

REMOVE SUPERSTRUCTURE: Include in the lump sum bid for "Remove Superstructure" all costs (materials, labor, equipment) associated with removing and disposing of the existing superstructure (including any wearing surface) and soil/backfill as necessary behind beams as detailed herein in accordance with Section 203 of the Specifications.

ELEVATIONS: The elevations given in these plans are relative elevations based on a point located on Abutment 1. The elevation at this location is assumed to be 100 feet and is not based on sea level elevations. Before starting any demolition, the contractor should make a reference benchmark off the bridge.

TYPICAL SECTIONS: Dimensions shown on the typical sections for pavement width and thickness are nominal or typical dimensions. The actual dimensions to be constructed may be varied to fit existing conditions as directed or approved by the engineer.

COMPACTION OF ASPHALT MIXTURES: Will accept the compaction of asphalt mixtures furnished on this project by option B according to subsections 402.03.02 and 403.03.10 of the standard specifications.



The following abbreviations may have been used in the preparation of these plans:

Bottom of Footing

between

Back Face

BOS Bottom of Slab bot. **Bottom** Brg. Bearing C to C Center to Center Current Edition c.e. C.Y. Cubic Yards Chd Chord Center Line CL Concrete Conc Cubic Cu. Drawing Dwa e.f. Each Face EI. Elevation Egual eq. Estimate Est. Ext. Exterior Face to Face F to F f.f. Front Face f.s. Far Side Front Feet I.D. Inside Diameter

bet.

b.f.

BOF

in. Inch Int. Interior

LBS Low Bridge Seat
LBS. Pounds

M Meter
MPH Miles Per Hour
n.s. Near Side
O.D. Outside Diamete

Opp. Opposite PC Point of Curvature

Perp. Perpendicular
Pl Point of Intersection

PPC Precast Prestressed Concrete
PPCDU Precast Prestressed Deck Unit
PSI Pounds per Square Inch

PT Point of Tangency

R Radius

RCBC Reinforced Concrete Box Culvert RCDG Reinforced Concrete Deck Girder

Rea'd Required RR Railroad Shld. Shoulder Spaces spa. Station Sta. Std. Standard Str. Straight Tan Tangent Thru Through Top of Footing

TOF Top of Footing
TOS Top of Slab
Tot. Total
Typ. Typical
Vert. Vertical
W.P. Working Point

COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS

REVISION
DATE

Division of Structural Design

DATE: DECEMBER 2024 CHECKED BY
DESIGNED BY: N. CORDTZ L. LIKINS
DETAILED BY: M. BAWITHAWNG N. CORDTZ

GENERAL NOTES

CROSSING
TRIB TO TOWN CREEK

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MATERIAL STEEL A.S.T.M AASHTO
High Strength Low Alloy A709 GR 50 M270 GR 50
Shear Stud Connectors UNS G 1018 M-169

Shear Stud Connectors UNS G 1
Sheet lead and Pig Lead B29-79

High strength bolts, nuts, and washers F3125 Grade A325 M-164 Type 1

All steel in longitudinal rolled wide flange beams shall meet the longitudinal Charpy V-Notch toughness test for non-fracture critical components Zone 2 in accordance with the following:

M270 GR 50 (up to 2" thickness) of 15 ft-lbs at 40°F.

Sampling and testing procedures shall be in accordance with AASHTO T243 current edition, utilizing (H) frequency testing. When plate thickness exceeds  $1\frac{1}{2}$ " frequency of testing shall be (P).

HIGH STRENGTH BOLT CONNECTIONS: Unless otherwise specified on the plans, all bolted connections shall be ASTM F3125 Grade A325  $\frac{3}{4}$ " diameter high strength bolts, nuts, and washers. Open holes shall be  $^{13}\!/_{16}$ " diameter. Type 1 galvanized bolts shall be used as described in AASHTO M164. All high strength bolted field connections are to be installed with "direct tension indicators" (DTI's) in accordance with the Standard Specifications and ASTM F959. All DTI's shall be manufactured from a steel conforming to the chemical requirements of ASTM A325 for Type 1 galvanized steel. DTI's shall be installed under the bolt head with the bumps facing the underside of the bolt head. Put a hardened washer under the nut and tension from the nut.

CORROSION PROTECTION: These beams and all steel components are to be hot dip galvanized according to ASTM A123. Weathering Steel is not allowed.

SHEAR CONNECTORS: The minimum length of studs is 4". Provide the necessary length to penetrate at least 2" above bottom of slab.

The "Lump Sum Bid" for shear connectors shall be full payment for all necessary shear connectors, welding and welding material, and materials necessary to field weld or shop weld the shear connectors in place according to the plans and specifications.

If the Contractor wishes to use something other than the stud shear connectors shown on the plans, the proposed arrangement shall be submitted for approval with the shop plans.

Studs shall be welded in accordance with AWS Specifications.

MILL TEST REPORTS: Notarized mill test reports shall be furnished in triplicate to the Department, showing that all material used in the structural steel conform to the requirements of the specifications.

PROHIBITED WELDING: No welding of any nature, other than indicated on the plans, is to be performed without the written consent of the designer, and then only in the manner and at the locations designated in the authorization.

SHOP DRAWINGS: The fabricator shall submit all required shop plans, by e-mail, to the design engineer for review. These submissions shall depict the shop plans in .pdf format. As either 11"x17" or 22"x36" sheets. Designers will make review comments on these electronic submissions as needed and shall return them to the fabricator. Upon reconciliation of the designers comments, files shall be returned to the designer and plans will be forwarded to the Division of Structural Designs Shop Plan coordinator for distribution. Only plans submitted directly to the shop plan coordinator will be distributed and only plans electronically stamped "Distributed by The Division of Structural Design" are to be used for fabrication. While this process does not require the submission of paper copies, The Division of Structural Design reserves the right to require such copies on a case by case basis.

When any changes to the design plans are proposed by the Fabricator or Supplier, the shop drawings reflecting these changes shall be submitted to the Design Engineer through the contractor.

PAYMENT FOR STRUCTURAL STEEL: The lump sum bid for structural steel shall be full payment for all structural steel, bolts, washers, paint, welding and welding materials, floor drains, bearings, and all labor and materials necessary to erect the steel in accordance with the plans and specifications. The approximate weight of structure steel shown in the estimate of quantities does not include overrun.

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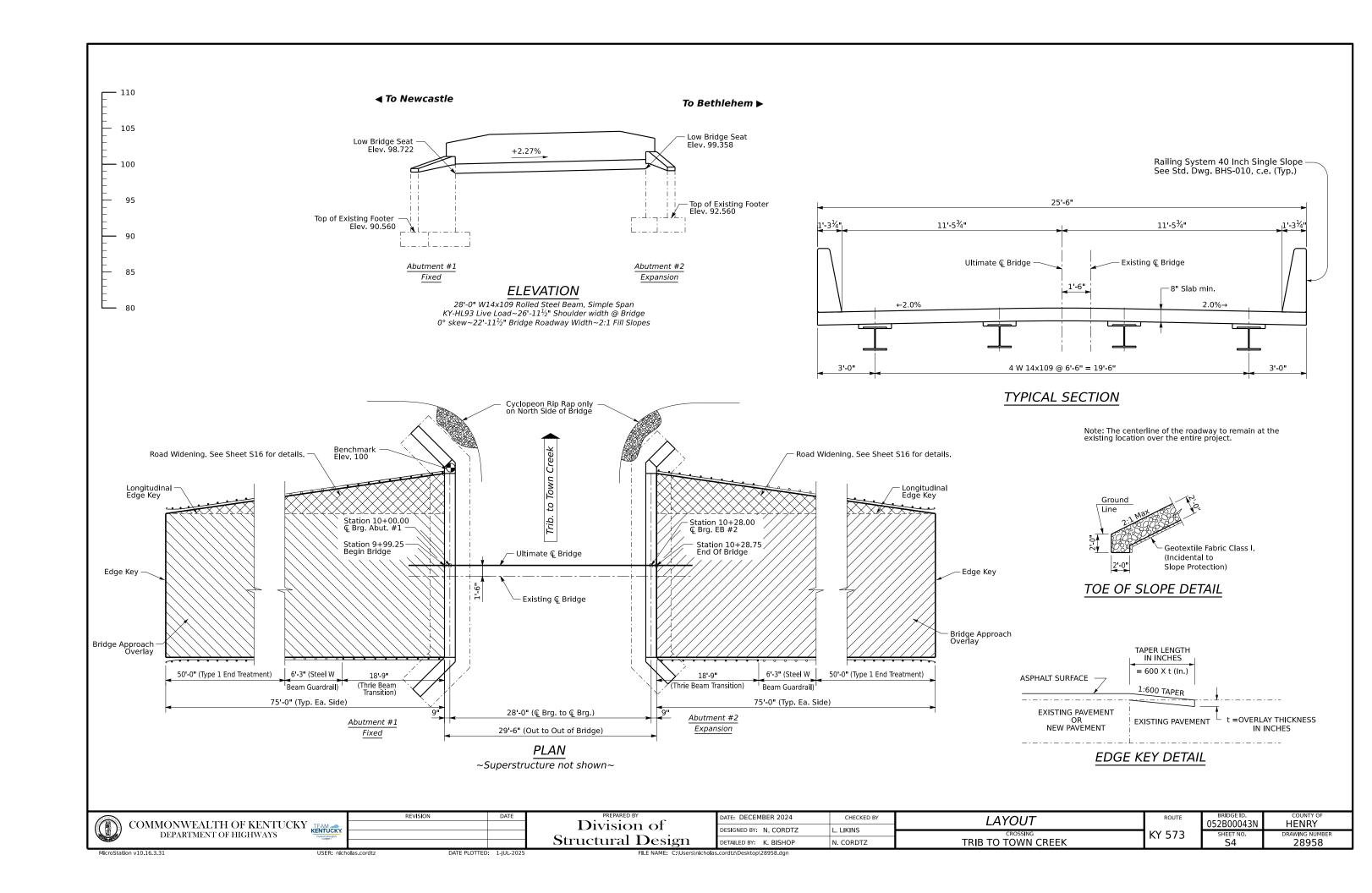
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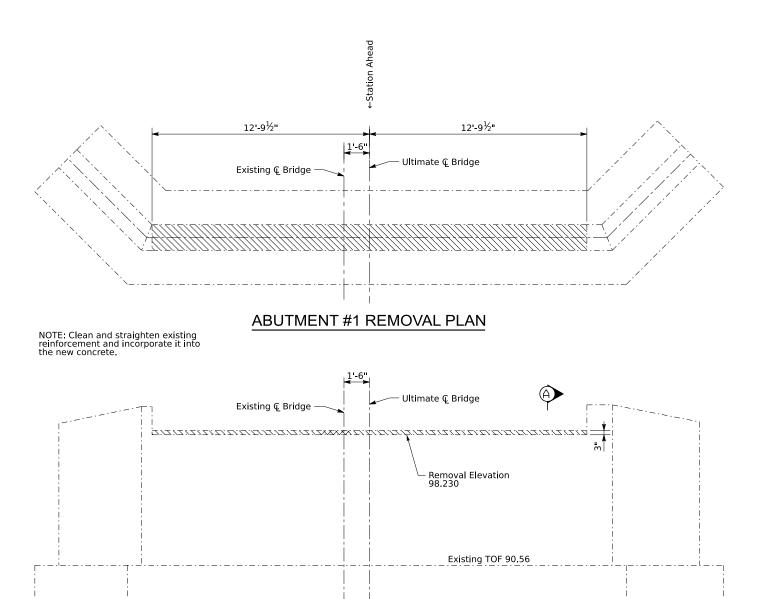
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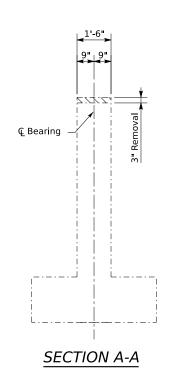
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Note: Remove cross-hatched portion of the existing concrete.



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Division of Structural Design

ABUTMENT #1 REMOVAL ELEVATION

 DATE:
 DECEMBER 2024
 CHECKED BY

 DESIGNED BY:
 N. CORDTZ
 L. LIKINS

 DETAILED BY:
 K. BISHOP
 N. CORDTZ

CONCRETE REMOVAL DETAILS

CROSSING
TRIB TO TOWN CREEK

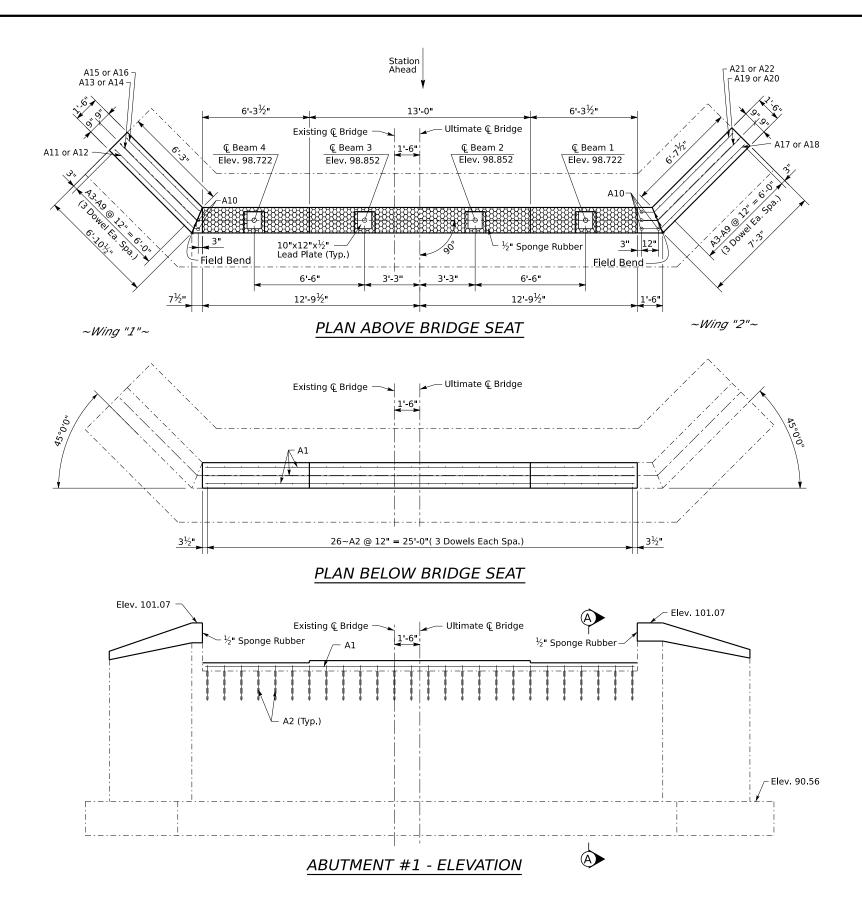
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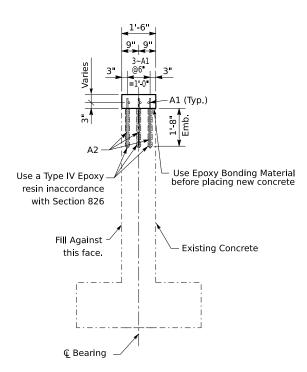
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NOTE: Field Bend and trim if necessary

NOTE: Ensure not to drill into existing rebar, adjust location if necessary.

NOTE: Beam elevations are given at the top of concrete.

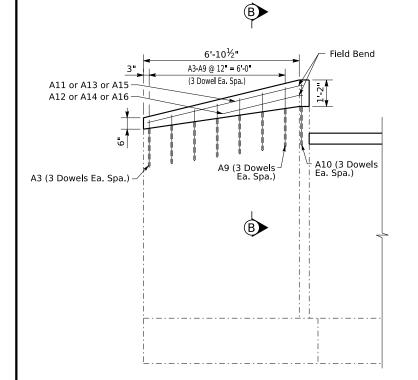


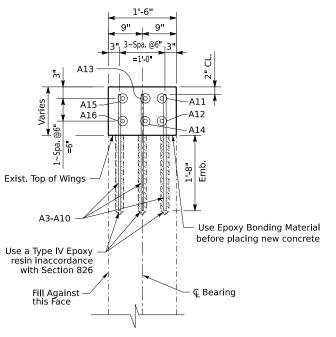
### SECTION A-A

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of Class "A" Concrete.

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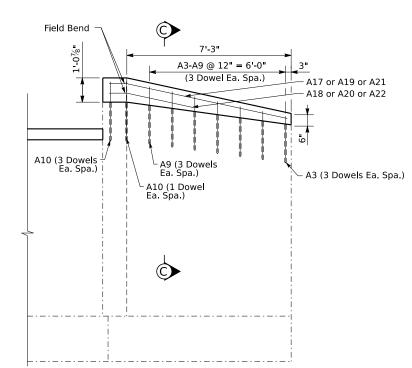
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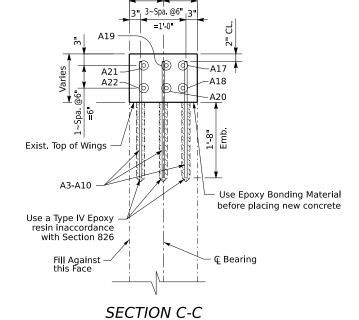


### SECTION B-B

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of Class "A" Concrete.



WING #1 - ELEVATION



### WING #2 - ELEVATION

### NOTE: The cost of

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of Class "A" Concrete.

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BILL OF REINFORCEMENT

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LOCATION

Breastwall

Wings Dowels

Wing 1

Wing 1

Wing 1

Wing 1

Wing 1

Wing 1

Wing 2

Wing 2

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HENRY
DRAWING NUMB
28958

Breastwall Dowels

SIZE LENGTH

MARK

A1

A2

A3

Α4

A8

Α9

A10

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A12

A13

A14

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A16

A17

A18

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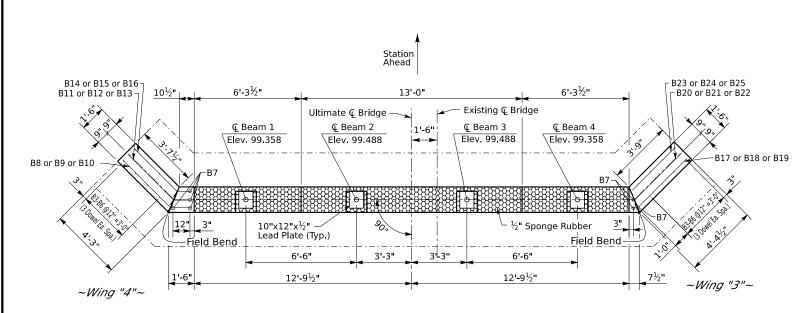
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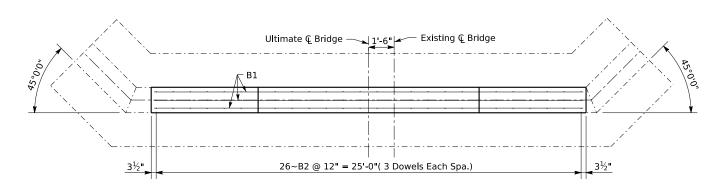
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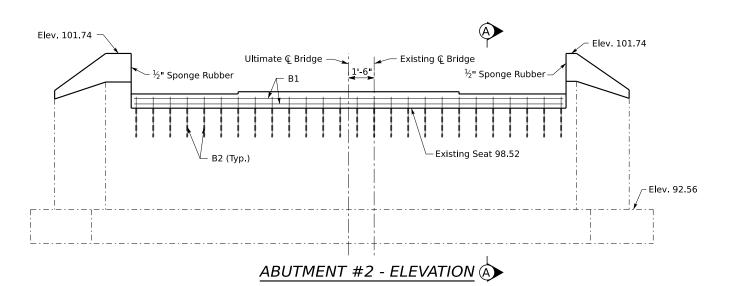
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### PLAN ABOVE BRIDGE SEAT



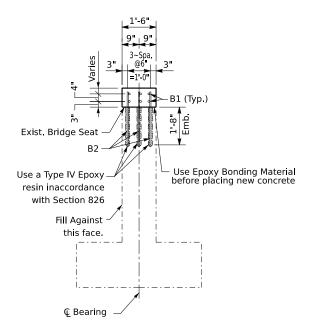
### PLAN BELOW BRIDGE SEAT



NOTE: Field Bend and trim if necessary

NOTE: Ensure not to drill into existing rebar, adjust location if necessary.

NOTE: Beam elevations are given at the top of concrete.

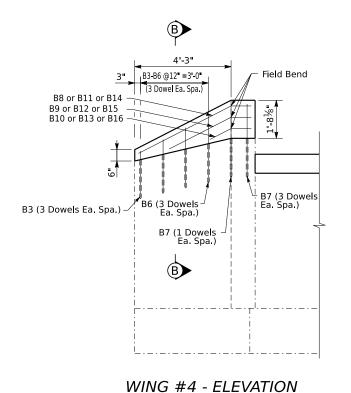


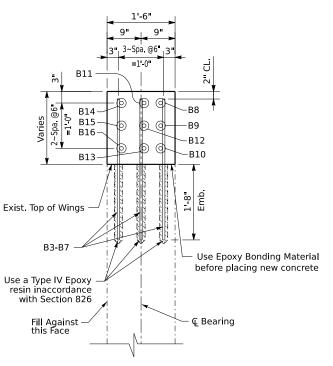
### SECTION A-A

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of Class "A" Concrete.

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COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS				DESIGNED BY: N. CORDTZ	L. LIKINS	CROSSING	KY 573	SHEET NO.	DRAWING NUMBER
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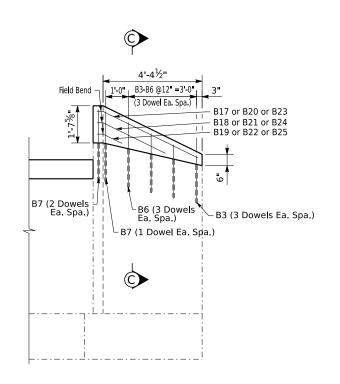
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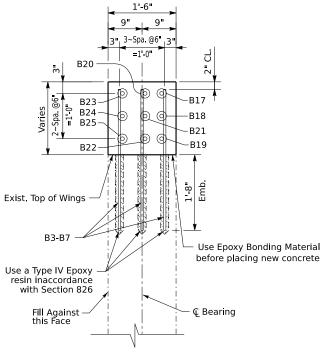




### SECTION B-B

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of Class "A" Concrete.





### WING #3 - ELEVATION

SECTION C-C

NOTE: The cost of drilling holes, grouting, and epoxy bonding material shall be incidental to the cost of

Class "A" Concrete.

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DETAILED BY: M. BAWITHAWNG	N. CORDTZ	TRIB TO TOWN CREEK		S9

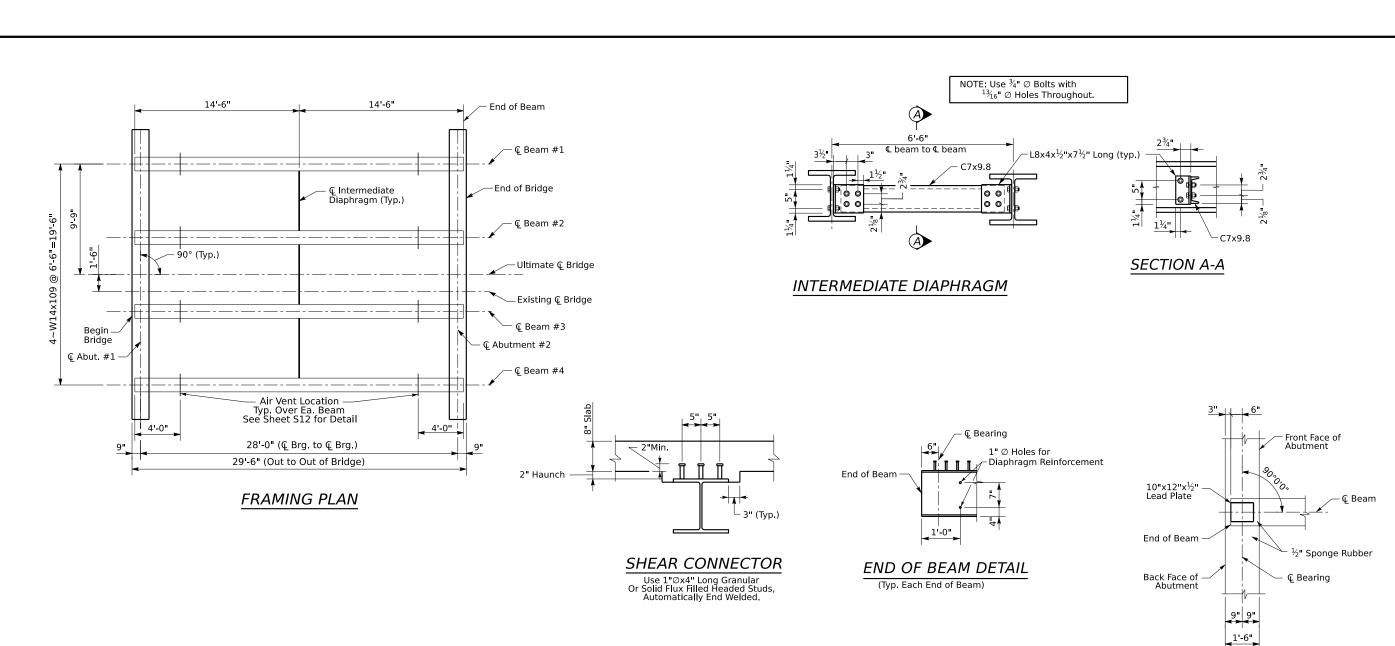
MicroStation v10.16.3.31 USER: nicholas.cordtz DATE PLOTTED: 1-JUL-2025 FILE NAME: C:\Users\nicholas.cordtz\Desktop\28958.dgn

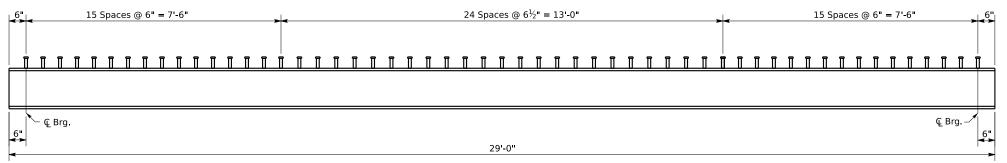
MARK TYPE NO. SIZE LENGTH LOCATION B1 Breastwall Breastwall Dowels ВЗ Str. Wings Dowels Wings Dowels Str. Wings Dowels Str. 2-10 Wings Dowels Str. Str. В8 5- 9 Str. Wing 4 B9 4- 3 Str. Wing 4 B10 Str. Wing 4 B11 Str. 5- 7 Wing 4 4- 1 B12 Str. Wing 4 B13 B14 Str. Wing 4 Wing 4 Wing 4 Str. 3-10 B16 Str. 1-10 Wing 4 B17 5- 0 Wing 3 Str. B18 3- 5 Str. Wing 3 B19 Str. Wing 3 4-10 B2Ø Str. Wing 3 Str. Wing 3 Str. Wing 3 4- 8 B23 Str. Wing 3 B24 3- 1 Wing 3 Wing 3

HENRY

28958

BILL OF REINFORCEMENT





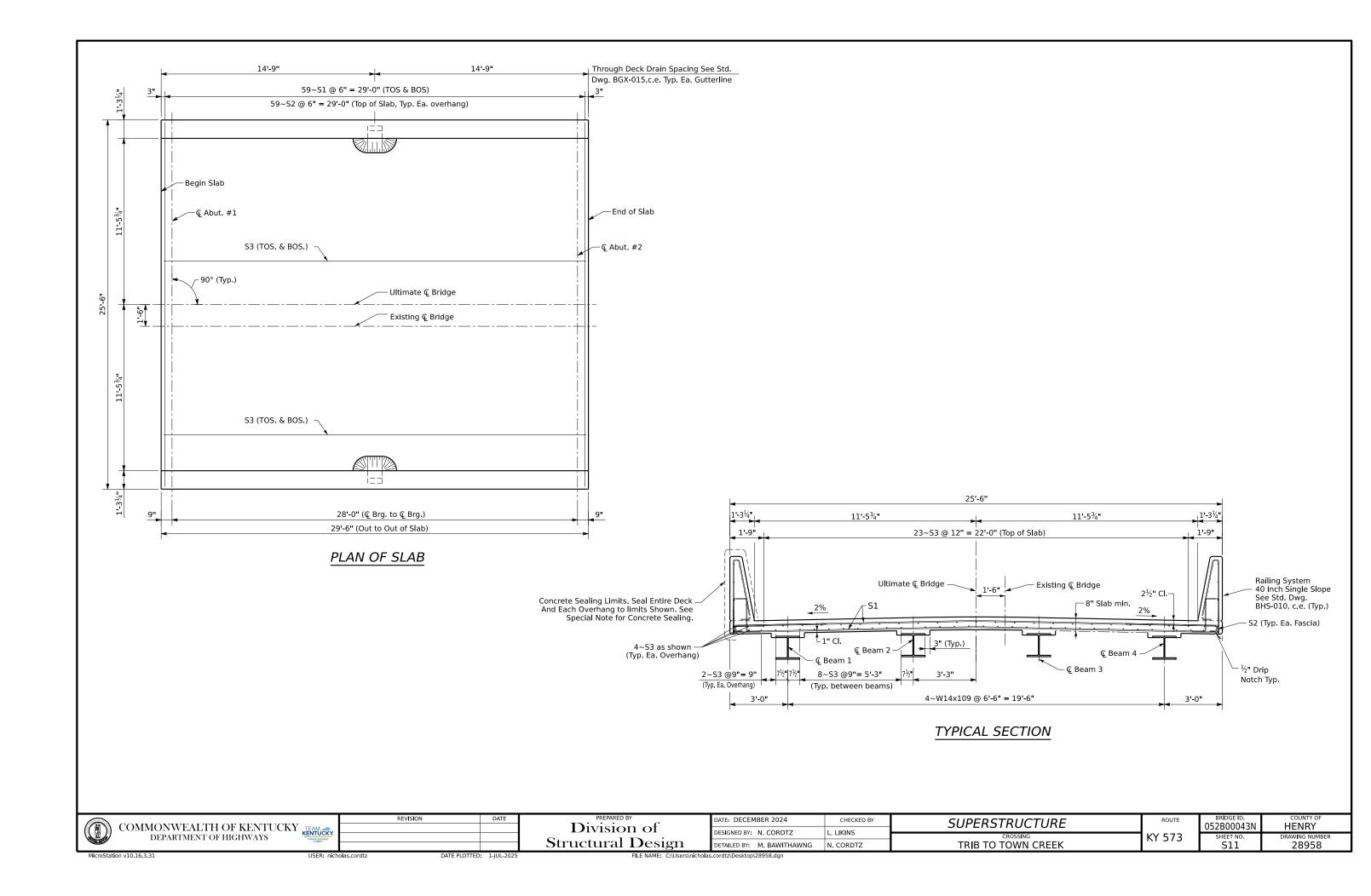
### END OF BEAM DETAIL @ ABUTMENTS

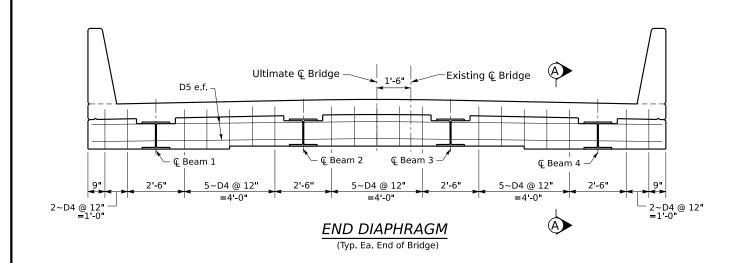
Use  $\frac{1}{2}$  " Sponge rubber between the diaphragm and the top of the support wall and between the diaphragm and the wings.

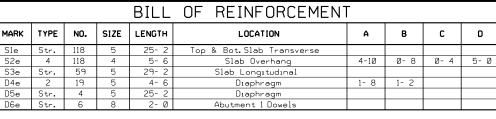
## GIRDER ELEVATION

Galvanize all steel according to ASTM A123
 Install shear studs before galvanizing

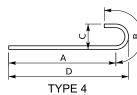
REVISION BRIDGE ID. 052B00043N COMMONWEALTH OF KENTUCKY DATE: DECEMBER 2024 CHECKED BY FRAMING Division of HENRY DESIGNED BY: N. CORDTZ L. LIKINS KY 573 DEPARTMENT OF HIGHWAYS AWING NUMB 28958 Structural Design TRIB TO TOWN CREEK DETAILED BY: M. BAWITHAWNG N. CORDTZ

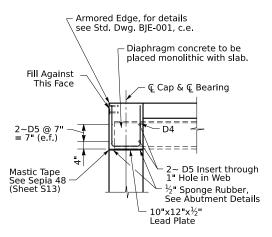








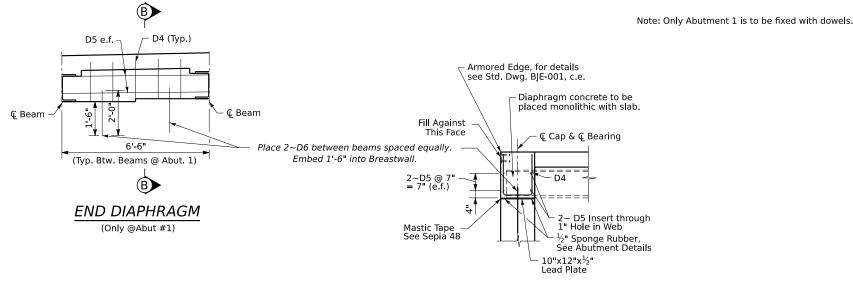




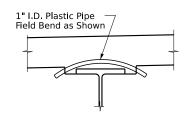
### SECTION A-A

Notes: 1.) Diaphragm stirrups are to project into the slab regardless of slab forming method.

2.) Place stirrup bars parallel to face of beams.



SECTION B-B



Note: Place 1" plastic pipe above beams 4'-0" from each end. Work and material is incidental to superstructure concrete.

AIR VENT DETAIL

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION DAT

Division of Structural Design

 DATE: DECEMBER 2024
 CHECKED BY

 DESIGNED BY: N. CORDTZ
 L. LIKINS

 DETAILED BY: M. BAWITHAWNG
 N. CORDTZ

SUPERSTRUCTURE

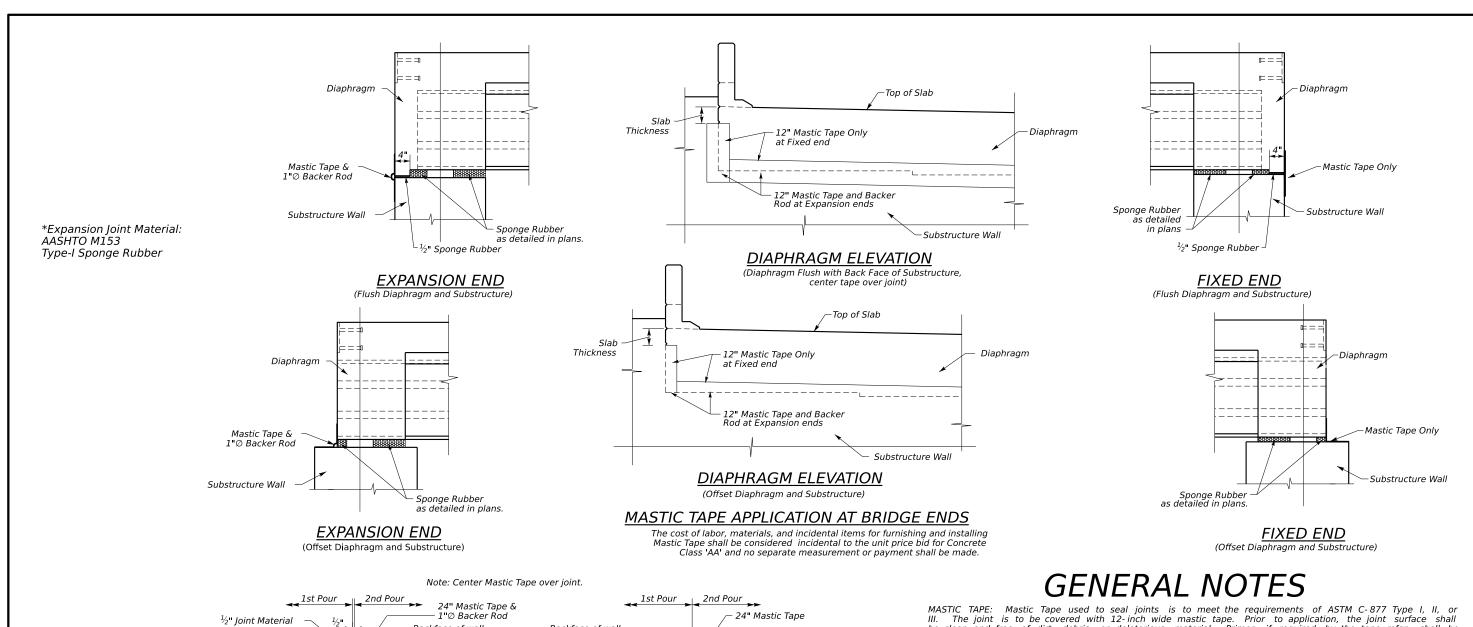
CROSSING
TRIB TO TOWN CREEK

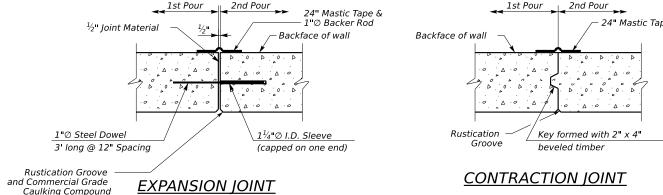
ROUTE

KY 573

BRIDGE ID. COUNTY OF 052B00043N HENRY SHEET NO. DRAWING NUMBER S12 28958

MicroStation v10.16.3.31 USER: nicholas.cordtz DATE PLOTTED: 1-JUL-2025 FILE NAME: C:\Users\nicho





Maintain 2 inch clearance from ends of Longitudinal reinforcement to edge of expansion joint.

(Color to match concrete)

### MASTIC TAPE APPLICATION AT RETAINING WALLS

MASTIC TAPE: Mastic Tape used to seal joints is to meet the requirements of ASTM C-877 Type I, II, or III. The joint is to be covered with 12-inch wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape mfgr., shall be applied for a minimum width of nine inches on each side of the joint.

Mastic Tape shall be either:

EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC. , CADILLOC by the UP RUBBER CO. INC. or approved equal.

Mastic Tape shall cover the joint continuously unless otherwise shown in the plans. Mastic Tape shall be spliced by lapping a minimum of six inches and in accordance with the mfgrs. recommendations with the

All preformed expansion joint material, caulking, mastic tape, pipe sleeve and equipment and labor necessary to complete the joints are incidental to the square foot bid for Retaining Walls.

REVISION DATE: DECEMBER 2024 COMMONWEALTH OF KENTUCKY CHECKED BY SEPIA 048 - IOINT WATERPROOFING 052B00043N Division of **HENRY** DESIGNED BY: N. CORDTZ . LIKINS DEPARTMENT OF HIGHWAYS KY 573 Structural Design TRIB TO TOWN CREEK DETAILED BY: M. BAWITHAWNG N. CORDTZ 28958

MicroStation v10.16.3.31 DATE PLOTTED: 1-IUL-2025

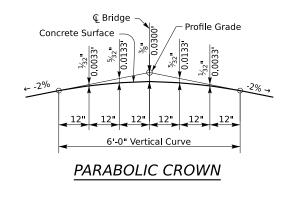
	CONSTRUCTION ELEVATIONS																
LOCATION	LEFT	BEAM 1			BEAM 2			<b>€</b> BRIDGE			ВЕАМ З			BEAM 4			RIGHT
	GUTTER	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	CONSTR.	TOP OF	DIM.	GUTTER
	GOTTER	ELEV.	BEAM	"X"	ELEV.	BEAM	"X"	ELEV.	BEAM	"X"	ELEV.	BEAM	"X"	ELEV.	BEAM	"X"	0011EI
SKEW LN AA	100.737	100.771			100.901			100.936			100.901			100.771			100.737
SKEW LN BB	100.754	100.788			100.918			100.953			100.918			100.788			100.754
SKEW LN CC	101.390	101.425			101.555			101.590			101.555			101.425			101.390
SKEW LN DD	101.407	101.442			101.572			101.607			101.572			101.442			101.407
GRID LN 01	100.908	100.943			101.071			101.106			101.071			100.943			100.908
GRID LN 02	101.100	101.135			101.262			101.296			101.261			101.134			101.100
GRID LN 03	101.272	101.306			101.434			101.469			101.434			101.306			101.272

### 6'-0" 3-Grid Lines @ 8'-0"=16'-0" 6'-0" (A) (B) (C) Lt. Gutter - & Beam 1 Begin-Bridge -End of Bridge 4~W 14×109 @ 6'-6"=19'-6" -Ç Beam 2 - Ultimate & Bridge - Existing & Bridge € Abut. #1 € Abut #2 C Beam 4 └─ Rt. Gutter (3) A B

28'-0" (© Brg. to © Brg.)

29'-6" (Out to Out of Bridge)

**GRID LAYOUT** 



### NOTES FOR ELEVATIONS TAKEN ON STEEL BEAMS

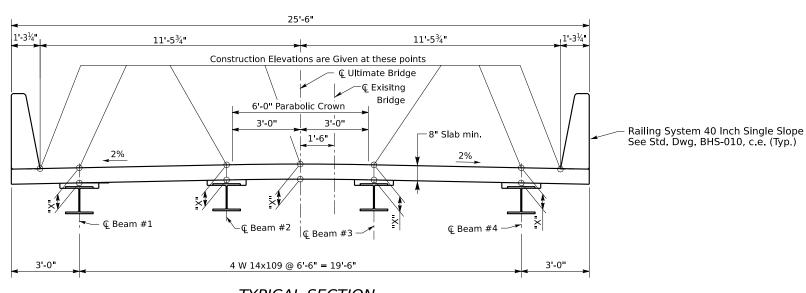
Take elevations on top of beam at points indicated by the grid layout. The beam elevations are to be read to three decimals, and entered in tables under "Top of Beam" elevations.

Compute dimension "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals dimension "X". Construction Elevations include camber due to weight of the concrete slab and barrier. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.

Construct barrier to roadway grade. Do not add camber to the barrier.



TYPICAL SECTION

REVISION

TEAM
KENTUCKY

MORPHOREM

Division of Structural Design

 DATE: DECEMBER 2024
 CHECKED BY

 DESIGNED BY: N. CORDTZ
 L. LIKINS

 DETAILED BY: M. BAWITHAWNG
 N. CORDTZ

CONSTRUCTION ELEVATION

CROSSING

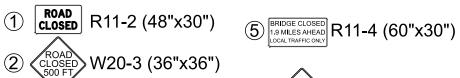
TRIB TO TOWN CREEK

ROUTE BRIDGE ID. 052B00043N KY 573 SHEET NO. S14

52B00043N HENRY

SHEET NO. DRAWING NUMBER

\$14 28958



- **DETOUR** M4-8P (24"X12")
- 17 WEST M3-4 (24"X12")

DETOUR W20-2 (36"X36")

M4-9 (30"x24")

(18) **EAST** M3-2 (24"X12")

ROAD W20-3 (36"x36")

 $\textcircled{4} \overset{\text{BRIDGE CLOSED}}{\underset{\text{LOGAL TRAFFIC ONLY}}{\text{BRIDGE TLOSED}}} R11\text{--4 } (60\text{"x}30\text{"})$ 

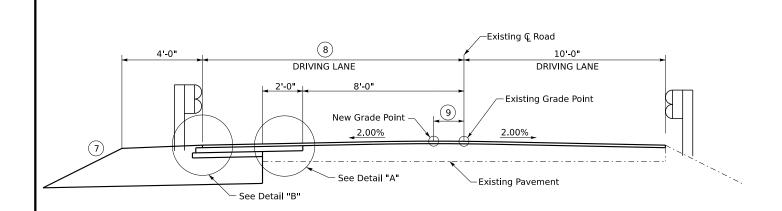
673 M1-5 (30"x24")

- M4-9 (30"x24")
- M4-8a (24"x18")



		REVISION	DATE	PREPARED BY	DATE: OCTOBER 2024	CHECKED BY	DETOUR	ROUTE	BRIDGE ID.	COUNTY OF
COMMONWEALTH OF KENTUCKY TEAM			Division of	DESIGNED BY: N. CORDTZ				052B00043N	HENRY	
	DEPARTMENT OF HIGHWAYS  KENTÜCKY			C 1.D	DESIGNED BY: N. CORD12	L. LIKINS	CROSSING	KY 573	SHEET NO.	Drawing Number
	CARRET			Structural Design	DETAILED BY: M. BAWITHAWN	G N. CORDTZ	TRIB TO TOWN CREEK		S15	28958

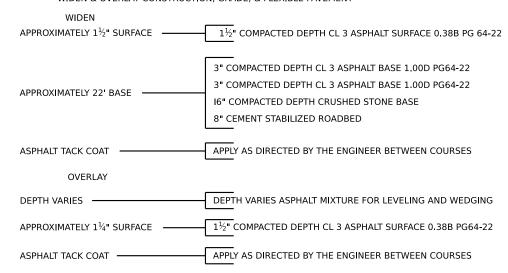
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Note: The centerline of the roadway is to remain at the existing location over the entire project,

WIDENING KY 573 STA. 9+24.25 to STA. 9+99.25 STA. 10+28.75 to STA. 11+03.75

### KY 573 WIDEN & OVERLAY CONSTRUCTION, GRADE, & FLEXIBLE PAVEMENT

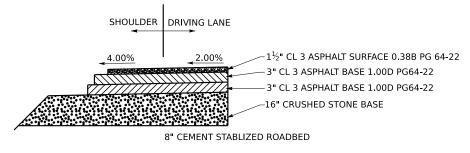


ASPH. CONC. SURFACE
ASPH. CONC. BASE

### TAPERING OF OVERLAYS ON MEDIUM SPEED FACILITIES (45 MPH to 65 MPH)

- MINIMUM COMPACTED THICKNESS
- (2) ASPHALT MIXTURE FOR LEVELING AND WEDGING OR NEXT COURSE OF ASPHALT MIXTURE.
- (3) ASPHALT SURFACE THICKNESS (FULL DEPTH)
- $\begin{tabular}{llll} \hline \bf 44 & MILL EXISTING PAVEMENT TO RECEIVE ASPHALT SURFACE FULL DEPTH (EDGE KEY). \\ \hline \bf TAPER LENGTH & (ft) = $t$ (in) $x$ TAPER RATE $$ \\ \hline \end{tabular}$
- (5) SAW CUT EXISTING PAVEMENT AS DIRECTED BY THE ENGINEER TO PROVIDE A UNIFORM EDGE TO ABUT NEW PAVEMENT AGAINST, SAW CUT SHALL BE NO LESS THAN 2,0' INSIDE THE PROPOSED EDGE OF PAVEMENT AND SHALL BE INCIDENTAL TO ROADWAY EXCAVATION
- (6) CONSTRUCT LONGITUDINAL EDGE KEY IN EXISTING PAVEMENT AS NECESSARY TO PROVIDE FOR A MINIMUM 3" LAYER OF ASPHALT BASE OVERLAY.
- 7 APPLY ASPHALT SEAL COAT FROM THE OUTSIDE EDGE OF THE PAVED SHOULDER TO A POINT 2' DOWN THE DITCH OR FILL SLOPE: TWO (2) APPLICATIONS AT THE RATE OF: 2.40 LBS/SY: ITEM 103 ASPHALT SEAL COAT: 20 LBS/SY: ITEM 100 ASPHALT SEAL AGGREGATE
- (8) TAPER FROM 10'-0" TO 12'-11¾" STA. 9+24.25 to STA. 9+99.25 TAPER FROM 12'-11¾" TO 10'-0" STA. 10+28.75 to STA. 11+03.75
- (9) TAPER FROM 0'-0" TO 1'-6" STA. 9+24.25 to STA. 9+99.25 TAPER FROM 1'-6" TO 0'-0" STA. 10+28.75 to STA. 11+03.75

Note: For a Taper Rate of 1 : 600 Taper Length = 75 feet when t= 1.5 inches



DETAIL "B"

DETAIL "A"

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION DA

Division of Structural Design 
 DATE: DECEMBER 2024
 CHECKED BY

 DESIGNED BY: N. CORDTZ
 L. LIKINS

 DETAILED BY: M. BAWITHAWNG
 N. CORDTZ

TYPICAL ROADWAY SECTIONS

CROSSING
TRIB TO TOWN CREEK

ROUTE BRIDGE ID. COUNTY OF 052B00043N HENRY

KY 573 SHEET NO. DRAWING NUMBER 28958

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